Intent

It is the intent of this specification to provide for installation of a 6 feet - 30 feet long tubular resin impregnated sleeve, affixed to the walls in the middle of a main sewer pipe run.

General

The method involves the impregnation of an absorbent carrier material; the inversion of the material into the pipe, the curing of the sleeve leaving behind a hard plastic sleeve mechanically bonded to the host pipe.

Reference specification

Tensile strength ASTM-D638
Flexural Strength ASTM D-790
Flexural Modulus ASTM D-790
Chemical Resistance ASTM D 543

Industry Specifications

ASTM F-1216- Standard Practice for Rehabilitation of Existing Pipelines and Conduits by the Inversion and Curing of a Resin Impregnated Tube

The carrier material

The carrier material shall be of fibrous absorbent composition tailored to achieve the following

1. Allow the migration of resin from its internal structure by compressing to a thickness of less than 90% of its uncompressed thickness under a pressure 1psi
2. The material must consist of non degradable fiber's such as polyester or polypropelene or Corrosion resistant fiberglass.
3. The material must have an abrasion resistant, chemically resistant, fully bonded coated surface in the lateral portion to ensure that on curing a smooth surface free from blemishes, pinholes or loose non wetted fibers.
4. Where fiberglass is used a surface veil or a layer of felt must be used to act as barrier to prevent osmosis or wicking of the strands.
5. The carrier material must be resistant to the resin used and must withstand any installation forces without losing its integrity.

The resin

The resin must be a thermosetting resin cured by either heat or chemically via the use of accelerators, or any other safe energy source which does not involve the use of electrical current within the main sewer, unless where evidence can supplied of the intrinsic safety.

The resin must give give sufficient working time above ground to enable impregnation of the fabric, but must cure to sufficient hardness to carry overburden loads within a maximum of 8 hours from time of insertion inside the pipe.

The resin used must have resistance to most chemicals to be found within a sewer system . As a minimum it must have resistance to the following chemicals at the following temperatures

Chemical Resistance

The resin must be resistant to the chemicals likely to be within the pipe and as a minimum must be resistant to the chemicals below.

The test is done in accordance with ASTM D 543.
Exposure should be for a minimum of one months at 73.4F. During this period the CIPP test specimen should lose no more than 20% of its initial flexural strength and flexural modulus when tested in accordance with Section 8 of this practice.

Chemical Solution

<table>
<thead>
<tr>
<th>Chemical</th>
<th>Concentration</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tap Water (ph 6-9)</td>
<td>100%</td>
</tr>
<tr>
<td>Nitric Acid</td>
<td>5%</td>
</tr>
<tr>
<td>Phosphoric Acid</td>
<td>10%</td>
</tr>
<tr>
<td>Sulfuric Acid</td>
<td>10%</td>
</tr>
<tr>
<td>Gasoline</td>
<td>100%</td>
</tr>
</tbody>
</table>

Characteristics of the Repair Sleeve

Starting Point: Anywhere within main run
Length in Main Pipe: Up to 30 feet
Time for cure 2 hours at 110 deg F
Thickness: 6mm or 7.5mm.
Wrinkle allowance: 10%
End thickness: Tapered down to 5mm
Resin Carrier Combination

The resin and the carrier fabric when cured must meet the following minimum criteria

Testing Method

ASTM-D 790

Flexural Strength 4500 psi
Flexural Modulus 250 000 psi

Testing

Visual Inspection

On completion of the work a CCTV survey should be carried out and the repairs must be verified as per below

1. There should be no evidence of water ingress within the liner.
2. The sleeve should be watertight internally. Where bends or other pipe deformities are present the maximum wrinkling allowed is 10% of the pipe diameter.

Pressure Testing

If required by the owners bid documents, testing may take place by placing two push rod plugs to the beginning and the end of the sleeve. Air is fed into the isolated area and pressure losses are monitored.

Test criteria and acceptable losses to be determined in advance by the engineer in consultation with the contractor

Design Considerations

The design is based on the assumption of a partially deteriorated pipe. The CIPP is designed to support the hydraulic loads due to groundwater, since the soil and surcharge loads can be supported by the original pipe. The groundwater level should be determined by the purchaser and the thickness of the CIPP should be sufficient to withstand this hydrostatic pressure without collapsing. The following equation may be used to determine the thickness required:

\[
P = \frac{2KE}{1 - \nu^2} \left(1 - \frac{C}{(SDR - 1)^3} \right)
\]

where:

- \( P \) = groundwater load, psi (MPa),
- \( K \) = enhancement factor of the soil and existing pipe adjacent to the new pipe (a minimum value of 7.0 is recommended where there is full support of the existing pipe),
- \( E_L \) = long-term (time corrected) modulus of elasticity for CIPP, psi (MPa)
- \( \nu \) = Poisson’s ratio (0.3 average),
- \( SDR \) = standard dimension ratio of CIPP,
- \( C \) = ovality reduction factor = \([1 - 0.01q/(1+0.01q)^2]\)
- \( q \) = percentage ovality of original pipe = 100 x (Mean Inside diameter - Minimum Inside Diameter) / Mean Inside Diameter

Using this formula we derive the following tables

<table>
<thead>
<tr>
<th>Diameter (inches)</th>
<th>Thickness (mm)</th>
<th>Max Pressure Head (ft)</th>
</tr>
</thead>
<tbody>
<tr>
<td>8</td>
<td>6</td>
<td>40</td>
</tr>
<tr>
<td>10</td>
<td>6</td>
<td>20</td>
</tr>
<tr>
<td>12</td>
<td>6</td>
<td>11</td>
</tr>
<tr>
<td>15</td>
<td>7.5</td>
<td>14</td>
</tr>
</tbody>
</table>

The table is based on \( K = 7, E = 125,000 \) psi (50 year strength figure), \( \nu = 0.3, C=0.64 \) (ie 5% ovality) and \( N=2 \)

Hydraulic Design Considerations

A CIPP liner can reduce the inside pipe diameter and may affect flow in small diameter pipes, but the reduction is usually more than offset by the improved flow characteristics. These compare favorably with an existing concrete pipe’s high Manning flow coefficient of about 0.015 or corrugated pipe Manning flow coefficient of 0.024. Reduced flow is insignificant in oval sewer pipes. Area reduction is only 0.6% at 5% ovalization and 2.4% at 10% ovalization

The analysis may be done using the Manning Equation
**Brief description**

Method statement

The pipe must be clean and free from debris, encrustation and standing water. Using a camera measure the length of felt tubing required.

Mix the resin and hardener required and pour the required amount of resin into the tube.

Attach a Vacuum pump to the other end of the tube and feed the material through the nip roller, spacers and conveyor system. Adjust the speed of impregnation to ensure thorough wet out on both top and bottom.

Carefully wind the felt liner into the tank ensuring tension at all times. Turn the trailing end of the bladder back on to the nose cone. Strap the tube to the cone using a ratchet strap.

Introduce regulated compressed air (Max 6psi) to the tank and allow sufficient felt tubing to extrude out. Evert the felt liner into either a clear plastic sheet or into a 1.5mm thick coated felt tubing.

Pull a bladder/packer through the center of the everted liner.

Attach the bladder packer to everted liner.

Pull both into the pipe and locate at the defective pipe section.

Inflate packer with air. Then replace with warm water to speed up the cure.

Once the sample piece above ground has cured. Deflate packer. Release water and pull packer out of position.
Preparatory Procedures

Cleaning of the Main Pipe

The manhole to manhole section of main pipe must be free of debris, obstructions, scale or any other material that reduces the effective bore of the pipe or loose material which may be pushed forward by the inversion process.

Prerepair Survey on Main Pipe

A videotaped survey must be done on the main run with a pan and tilt camera to confirm the proposed repair falls within the limitation parameters set by the manufacturer on the following aspects; The potential flows coming through lateral pipes; the diametric size of the pipe for total chainage of the repair; the presence of active infiltration within the vicinity of the repair area; the weather; the identification of access to the head of each lateral connection; the exact metreage for the start and finish of the repair, the exact position of the lateral connections.

The Repair Process

On completion of the survey, a report should be submitted to the Engineer confirming the feasibility and the required program of works.

Post Repair Survey

On completion of a given run, video taped evidence must be provided by the contractor.

The repair sleeve should be monitored for excessive wrinkling, exposed unwetted fibers and pinhole leaks.

Testing

Subject to the result of the Inspection test, the Engineer reserves the right to select approximately 10% of the repairs for further testing using a water. The test shall comprise of pushing a plug to the downstream end of the sleeve, inserting a bypass plug at the upstream end and filling the isolated section with water. Losses shall be measured and must be within levels set by the local code.

If the failure rate exceeds 10% and additional retest area of an equivalent size shall be selected for further testing.

Quality Assurance

The workmanship of the works shall be warranted for a period of 12 month following substantial completion. The material shall be warranted as per terms and condition of the manufacturer for a similar period PROVIDED the contractor applies the product in accordance with the current guidelines (Standard Operating Procedure) set out by the manufacturer.

Typical Bill Items

<table>
<thead>
<tr>
<th>Description</th>
<th>Measurement</th>
<th>Unit Rate</th>
</tr>
</thead>
<tbody>
<tr>
<td>Main Pipe Cleaning</td>
<td>Per Run</td>
<td>General Contractor</td>
</tr>
<tr>
<td>Pre repair CCTV of Main pipe and mapping of laterals.</td>
<td>Per run</td>
<td></td>
</tr>
<tr>
<td>Mobilization of Repair Equipment</td>
<td>Lump sum</td>
<td></td>
</tr>
<tr>
<td>Set up for Installation of Liner</td>
<td>Each</td>
<td></td>
</tr>
<tr>
<td>Installation of 8” liner designed to withstand an external hydrostatic pressure of 10 feet.</td>
<td>ft</td>
<td></td>
</tr>
<tr>
<td>Post repair survey of pipe Black and White survey of main pipe.</td>
<td>Per run</td>
<td></td>
</tr>
<tr>
<td>Set up of equipment for reconnecting lateral</td>
<td>Per run</td>
<td></td>
</tr>
<tr>
<td>Cost of reconnecting each lateral</td>
<td>Each</td>
<td></td>
</tr>
</tbody>
</table>